

RCC Design:-

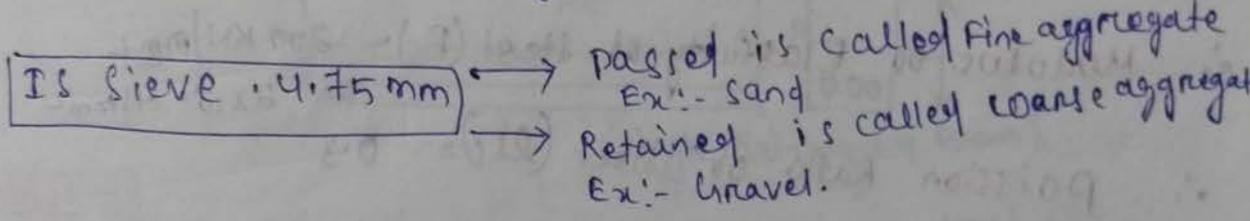
1. Concrete :- Cement Mixture of Cement, Coarse aggregate, Fine aggregate, water

Cement - Binder

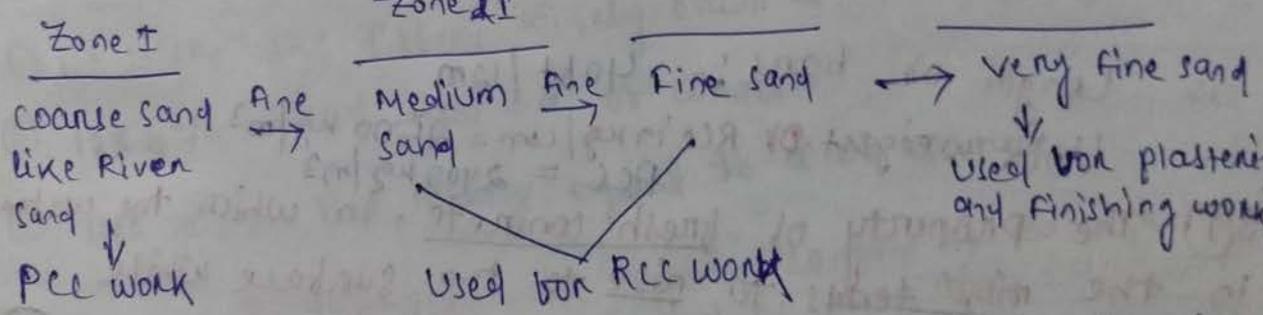
Coarse aggregate - strength

Fine aggregate - Void filler

Water - Workability + Hydration



Sand :- It has 4 Zone.



Water :- pH value of water shall not be less than 6.

permissible limit for solids in water:-

- (i) Organic matter = 200 mg/l or PPM
- (ii) Inorganic matter = 3000 mg/lit
- (iii) Sulphates = 400 mg/l
- (iv) Chlorides = 2000 mg/l for PCC, 500 mg/l for RCC
- (v) Suspended matter = 2000 mg/lit

Reinforcement (Steel) :- MILD STEEL - Fe 250

→ HYSD STEEL (High Yield Strength deformation bar)
[Fe 415, Fe 500, Fe 550]

→ Mild Steel code book = IS 432 (PART-1) → HYSD bar code book = IS 1786

→ Surface of mild steel is plane

→ Surface area of HYSD bar are angular.

Reinforcements are available in 8mm, 10mm, 12mm, 16mm, 20mm, 25mm, 32mm, 36mm.

→ Building → Fe 415

→ Govt project = Fe 500, Fe 550

∴ Modulus of Elasticity of steel (E_s) = 200 KN/mm² or 2×10^5 N/mm²

∴ Poisson Ratio of steel (μ) = 0.3

∴ Unit mass of steel = 7850 kg/m³ or 7.85 ton/m³

∴ Co-efficient of Thermal Expansion $\alpha = 12 \times 10^{-6} / ^\circ\text{C}$

∴ Length of bars :- 40ft / 12m

Unit weight of RCC in kg/cm = 2500 kg/m³
PCC = 2400 kg/m³

Q1) The property of fresh concrete, in which the water in the mix tends to rise to the surface while placing and compacting, is called.

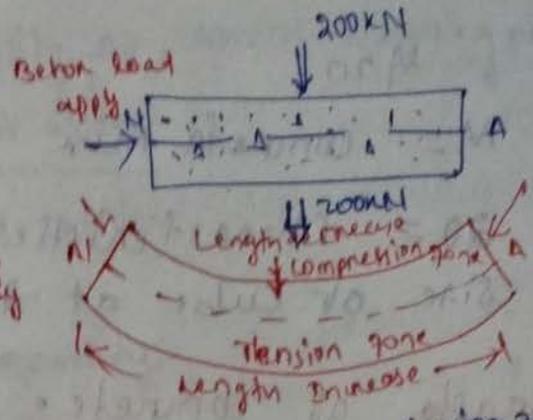
Ans (B)
a) segregation b) bleeding c) bulking d) creep

Q2) The property of the ingredients to separate from each other while placing the concrete is called.

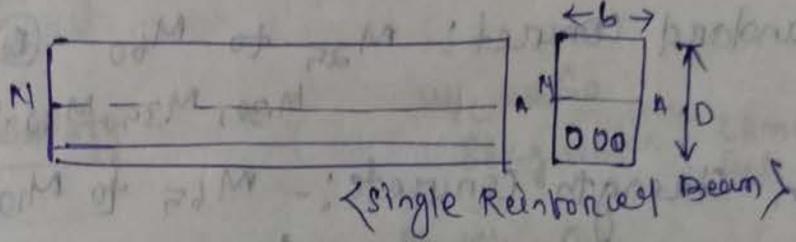
Ans (A)
a) Segregation b) compaction c) Shrinkage d) bulking

PCC (plain cement concrete)

→ Load carrying capacity = 100kN



Concrete weak in tension zone.
∴ We use provide reinforcement in tension zone. to strengthen in the beam. Reinforcement resist the additional 100kN load



class-2

1 Q) The thermal expansion co-efficient of steel is: - Ans C

- (a) $12 \times 10^{-6}/^{\circ}C$ and closely resembles to α of concrete.
- (b) $11 \times 10^{-6}/^{\circ}C$ and differs widely from α of concrete.
- (c) $12 \times 10^{-6}/^{\circ}C$ and close to α of concrete.
- (d) $14 \times 10^{-6}/^{\circ}C$ but nearly equal to α of concrete.

2) As per IS:456-2000, the organic content of water used in making concrete should not be more than. Ans A

- (a) 200 mg/lit
- (b) 250 mg/lit
- (c) 100 mg/lit
- (d) 150 mg/lit

3) Mild steel used in RCC structure conforms to: - Ans A

- (a) IS:432
- (b) IS:1566
- (c) IS:1786
- (d) IS:2062

4) Unit weight of RCC in kg/cm³ is: - Ans 2500

- (a) 1200
- (b) 1800
- (c) 2400
- (d) 3000
- (e) NOT

5) The grade of concrete approximately mix. Ans A

- (a) 1:3:6
- (b) 1:1:2
- (c) 1:2:4
- (d) 1:1.5:3
- (e) 1:1.5:3

M20

M = concrete mix

20 = characteristic compressive strength of 150mm size of cube at 28 days in 20 N/mm^2 .

Grade of concrete:-

① Ordinary concrete :- M10, M15, M20 ②

② Standard concrete :- M25 to M60 ③

M30, M35, M40, M45, M50, M55

③ High strength concrete :- M65 to M100 ④

(M65, M70, M75, M80, M85, M90, M95, M100) = 8 nos

Workability:- ~~work~~ The ease with which the fresh concrete can be mixed, transported, placed and compacted is called as workability of concrete.

Factors of workability:-

(i) water cement ratio $\left(\frac{w/c}{f}\right)$ It is ratio of mass of water (w) to the mass of cement (c).

$$\frac{w}{c} = \frac{w}{c}$$

Water cement ratio are generally lies in the interval $[0.40 \text{ and } 0.60]$

\Rightarrow M10, M15 & M20 $w/c = 0.40 \text{ and } 0.60$

M20 the commonly use water, cement ratio = $[0.45 \text{ to } 0.55]$

M10 concrete min quantity of cement = 220 kg per cum

Given (w/c)

$$w/c = \frac{\text{Quantity of water}}{\text{Quantity of cement}}$$

$$\Rightarrow 0.40 = \frac{220}{\text{Quantity of water}}$$

$$\Rightarrow 0.40 \times 220 = \text{water quantity}$$

$$\Rightarrow \frac{40}{100} \times 220$$

$$\Rightarrow 88 \text{ kg} = \text{water} \text{ m}$$

Required 88 kg water for M10 1 cum.

Workability

w/c ratio

88 kg water [Cement 220 kg
w/c = 0.4]

if w/c = 0.2

if workability \uparrow then workability \uparrow

$$\frac{88}{220} \times 0.2 \times 220 = \text{water reqd}$$

$$\Rightarrow \frac{2}{10} \times 220$$

\Rightarrow 44 kg water.

workability \uparrow w/c \uparrow
workability \downarrow w/c \downarrow

Time \uparrow workability \downarrow
Time \downarrow workability \uparrow

(i) Workability \propto w/c ratio

(ii) Workability \propto $\frac{1}{\text{Time}}$

(iii) Workability $\downarrow \propto \frac{1}{\text{Aggregate-cement Ratio}}$

workability \propto $\frac{1}{\text{Aggregate-cement Ratio}}$

(iv) Workability \propto grading of aggregate

Q) Which IS code provides recommended guidelines for concrete mix design?

- (a) IS 12813
- (b) IS 800
- (c) IS 1373
- (d) IS 10262

Ans (D)

It heavy loaded structure like overbridge, power plant, flyover, river bridge required more strength like 35 N/mm^2 , 40 N/mm^2 , 30 N/mm^2 .

After M_{10} , M_{20} , after M_{20} all concrete mix design are designed by IS code IS:10262.

Q) pick up the correct statement from the following.

Drying shrinkage is affected by.

- (a) The relative humidity of the atmosphere when the concrete is placed.
- (b) The length of time.
- (c) The water/cement ratio of the concrete.
- (d) All options are correct.

Ans (D)

Concrete shrinkage :- The volumetric change of the concrete structure due to loss of capillary water by evaporation is known as concrete shrinkage.

Factors :- Time, Relative humidity, w/c Ratio, type of aggregate, admixture, composition & fineness of cement.

class - 3

Involving stress method :-

1. Min^m grade for PCC = M15 } concrete under Normal condition.
 Min^m grade for RCC = M20

2. If concrete is in seawater or nearer to sea water or water bodies.

Min^m grade for PCC = M20
 Min^m grade for RCC = M30

3. Grade of concrete (IS 456-2000) :-
 Latest water Amendment No-4, 2013

Ordinary grade = M10, M15, M20
 Standard grade = M25, to M55
 High strength grade = M60 to M80

4. Concrete mix proportioning :-

- (i) Nominal mix concrete = user on site work (IS 456:2000) Mix directly on site
- (ii) Design mix concrete = mix only on batching plant (IS 10262) user on site work

5. properties of concrete :-

(i) Compressive strength :-

cube samples	Cylindrical Sample
Size = 150x150x150	Site = d = 150mm L = 300mm
After applying load	First cylindrical sample fail.

→ Compressive strength measured by compressive testing machine (CTM) or UTM (Universal Testing Machine).

⇒ cube has more frictional resistance as compare to cylinder
 ⇒ that why it doesn't fail easily.

(i) Cube sample strength = 1.25 time or strength of cylindrical sample.

(ii) 0.8 times of cubical sample = cylindrical sample strength.

⇒ cylindrical sample's $\frac{L}{D}$ Ratio = $\frac{300}{150} = 2$ or

Compressive strength of concrete:- Mpa or N/mm²

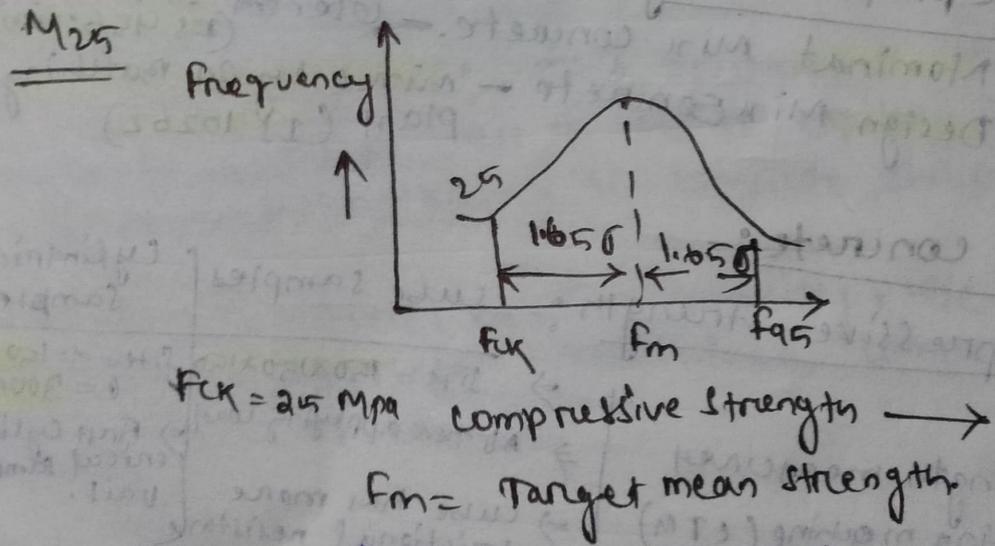
M20, $F_{ck} = 20$ N/mm² at 28 days.

Take	M25	Cube sample	cube 1	2	3	4	5	6	7
			17.5 mpa	19.5 mpa	21.5 mpa	25 mpa	28 mpa	29 mpa	30 mpa
		after 28 days.	↓	↓	↓				
		Compressive strength	Cube fail	Cube fail	Cube fail				
		become =	17.5 < 25 mpa	19.5 < 25 mpa	21.5 < 25 mpa				
			$F_{ck} = 25$ N/mm ²						

Not more than **5%** test Result are likely to be **fail**

95% test Result ^{of cube test} passed above **25** N/mm²

Normal distribution curve:- strength of concrete.



$F_{ck} = 25$ Mpa Compressive strength →
 $F_m =$ Target mean strength

$F_m = F_{ck} + 1.65\sigma$

Standard deviation:-

For	M10 & M15	$\sigma = 3.5$
	M20 - M25	$\sigma = 4.0$
	M30 - M50	$\sigma = 5.0$

Mathematical formula,
$$s = \sqrt{\frac{\sum_{i=1}^n (x_i - \bar{x})^2}{n-1}}$$

x_i = Individual sample

\bar{x} = Mean sample

n = No. of sample

Q. Avg. strength of concrete is 31.6 Then calculate the characteristic strength of concrete if $\delta = 4$

Solⁿ Avg strength = Target mean strength = 31.6 mpa

$$f_m = f_{ck} + 1.65 \delta$$

$$\Rightarrow 31.6 = f_{ck} + (1.65 \times 4)$$

$$\Rightarrow 31.6 - 6.60$$

$$\boxed{f_{ck} = 25 \text{ mpa}} \quad \underline{Ans}$$

Q. Avg strength of concrete is 38.25 mpa, then calculate characteristic strength of concrete if $\delta = 5$.

Solⁿ

$$f_m = 38.25$$

$$\Rightarrow f_m = f_{ck} + 1.65 \delta$$

$$\Rightarrow 38.25 = f_{ck} + 1.65 \times 5$$

$$\Rightarrow 38.25 - 8.25$$

$$\Rightarrow \boxed{30 \text{ mpa} = f_{ck}} \quad \underline{Ans}$$

Cube sample :-

1 days

at 28 days strength. 16%

3 days

40%

7 days

2/3th of 28 day strength. (65%)

14 days

90%

28 days

99%

after 1 year as compare to 28 days. = 20 to 25%

Class-4

Q) What is the 7 days strength of concrete if the 28 day strength is 25 mpa. (a) 13.3 (b) 16 (c) 17 (d) 19.5

Soln

$$F_{ck} = 25 \text{ mpa}$$

$$\begin{aligned} \text{7 day strength} &= \frac{2}{3} \times 28 \text{ days strength} \\ &= \frac{2}{3} \times 25 = \frac{50}{3} = 16.67 \text{ Mpa} \approx 17 \text{ mpa} \end{aligned}$$

Tensile strength of concrete:-

1. Tensile strength in flexure or bending. It is also called modulus of rupture.

$$F_{cr} = 0.7 \sqrt{F_{ck}}$$

2. In ~~direct~~ splitting strength $f_{cs} = \frac{2P}{\pi DL}$
 $= 0.67 \sqrt{F_{ck}}$

$$f_{cs} = \frac{2}{3} F_{cr}$$

3. In direct tension $= F_{ct} = 0.50 \sqrt{F_{ck}}$

$$\therefore f_{cr} > f_{cs} > f_{ct} *$$

Young's Modulus of Concrete:-

E_c or Modulus of Elasticity of concrete (short term)

$$E_c \Rightarrow 5700 \sqrt{F_{ck}} \text{ as per IS 456:1978}$$

$$E_c \Rightarrow 500 \sqrt{F_{ck}} \text{ as per IS 456:2000.}$$

\therefore For all practical purposes, modulus of elasticity of concrete is taken as Secant modulus.

Modulus of Elasticity of concrete (Long term)

$$E_L = \frac{E_0}{1+\theta}$$

$$E_L = \frac{5000\sqrt{f_{ck}}}{1+\theta}$$

θ = Creep co-efficient

$$\theta = \frac{\text{Creep strain}}{\text{Elastic strain}}$$

$$\theta = \frac{\text{Creep strain}}{\text{Elastic strain}} \text{ OR } \frac{\text{Ultimate creep strain}}{\text{Instantaneous Elastic strain}}$$

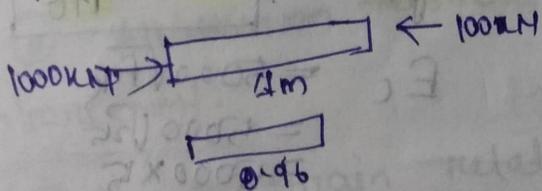
creep co-efficient value as per age of concrete at the time of loading.

$\theta_1 =$	7 days	2.2
	28 day	1.6
	1 year	1.1

$$\theta = \frac{\text{Creep strain}}{\text{Elastic strain}}$$

$$\text{Strain} = \frac{\text{change in length}}{\text{original length}}$$

Creep strain :- It is a time dependent strain in concrete due to sustained loading or permanent loading.



$$\epsilon = \frac{\Delta L}{L} = \frac{1-0.96}{1} = \frac{0.04\text{m}}{1000\text{mm}}$$

It load is applied 80 years on long period.

$$\text{Strain} = \text{Total deformation} = 40\text{mm} = 0.04\text{m}$$

$$40\text{mm} = \text{Elastic strain} + \text{creep strain}$$

$$\Rightarrow 40\text{mm} = 30\text{mm} + \text{creep strain}$$

$$\text{Creep strain} = 40 - 30 = 10\text{mm} = 0.01\text{m}$$

After removal of load creep strain not comes again.

\therefore Creep strain = permanent permanent deformation.

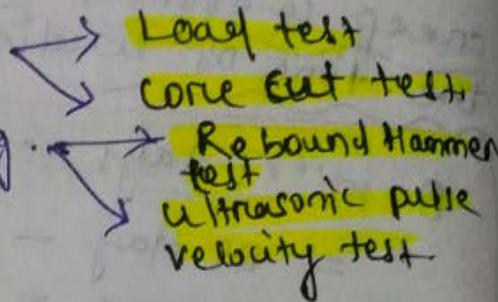
Creep can be controlled by:-

1. Using high strength of concrete.
2. Adding steel against it.
3. Steam curing under pressure.
4. Delaying the application of finishes.

Testing of hardened concrete:-

① Destructive Testing.

② Non-destructive Testing.



Q) For M25 grade concrete with creep co-efficient 1.5 the long term modulus of elasticity in Mpa as per IS 456: 2000.

Soln

M25 = $f_{ck} = 25 \text{ MPa}$

Modulus of elasticity (Long term) = $E_L = \frac{E_c}{1 + \theta}$

$$E_c = 5000 \sqrt{f_{ck}}$$

$$= 5000 \sqrt{25}$$

$$= 5000 \times 5$$

$$= 25000 \text{ MPa}$$

$\theta = 1.5$

$$E_L = \frac{25000}{1 + 1.5} = \frac{25000}{2.5}$$

$$= 10000 \times 10$$

$E_L = 10,000 \text{ N/mm}^2$

Q. The flexural tensile strength of M30 grade of concrete is N/mm^2 , as per IS 456-2000.

Solⁿ Flexural tensile strength $f_{cr} = 0.7 \sqrt{f_{ck}}$

M30, $f_{ck} = 30$

$$= 0.7 \sqrt{30}$$

$$= \frac{7}{10} \times 30 = 21 \text{ MPa}$$

$$= \frac{7}{10} \times 5.4$$

$$= \frac{38.8}{10} = 3.8 \text{ MPa}$$

Design philosophies :-

- ① Working stress method (WSM)
- ② Limit stress method (LSM)
- ③ Ultimate load method (ULM)

① WSM (Working stress Method)

Assumption :-

- ① plane section before bending will remain plane even after bending.
- ② All tension **tensile stresses** are taken up by reinforcement.

③ **stress-strain relationship of steel & concrete under working load is a straight line.**

④ **Modular Ratio** = $\frac{m}{1} = \frac{280}{3 \sqrt{f_{ck}}}$

f_{ck} = permissible stress in concrete in bending compression.

⑤ concrete & steel are well bonded together. **concrete thermal expansion = 12×10^{-6} / °C concrete = 10×10^{-6} / °C**

⑥ **Young's modulus of concrete = $5000 \sqrt{f_{ck}}$**

Value of f_{bc} (permissible stresses in concrete) : Task no 21

Grade of concrete	permissible stress in compression		permissible stress in Bond for plain Bars in tension.
	Bending	Direct	
Grade	f_{bc}	f_{cd}	f_{bd}
M15	5.0	4.0	0.6
M20	7.0	5.0	0.8
M25	8.5	6.0	0.9

Q: What is the modular Ratio of M25 or grade of concrete.

Sol: M25 $f_{ck} = 25 \text{ mpa}$

$$f_{cbc} = 8.5$$

Modular Ratio (m) = $\frac{280}{3f_{cbc}} = \frac{280}{3 \times 8.5} = \frac{280}{25.5} = 10.98$

$\frac{5.49}{16.17} \times \frac{280 \times 10^2}{85 \times 3} = 10.98$

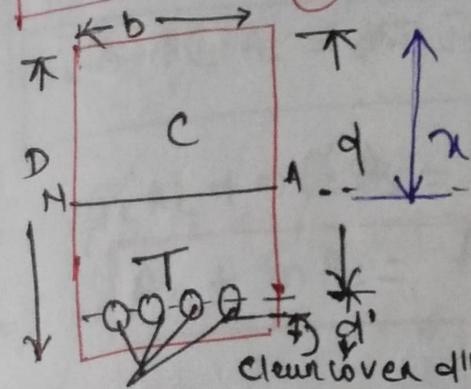
m = 10.98 Ans

permissible stress in steel (f_{st}) :

TB-22

Type of stress in steel reinforcement	Mild steel	HYSD bar (Fe415)
(i) Tension (f_{st})	140 N/mm ²	230 N/mm ²
(ii) upto and including 20mm	130 N/mm ²	230 N/mm ²
(iii) over 20mm	130 N/mm ²	190 N/mm ²
(iv) Compression in column bars (f_{sc})		

class-6



Asst Rectangular beam
 (balanced beam)
 b = width of beam.
 D = Overall depth.
 d = Effective depth.
 d' = Effective cover

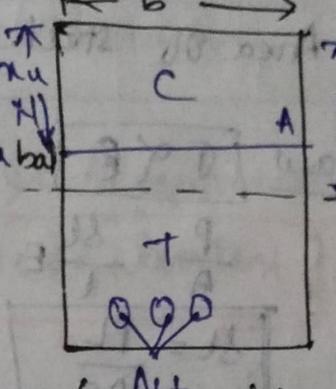
$D = d + d'$

d' = Distance betn outer most tension fibre to C.M of Tension steel.

A_{st} :- Area of steel in Tension zone

C = compression
 T = Tension zone.
 x_u = Depth of neutral axis.
 x_{ubal} = Depth of neutral axis at balance secn.

(b)



Asst Under Reinforced secn

(a) Balanced secn :-

$A_{st} = A_{st\ bal}$
 $x_u = x_{ubal}$ *

\Rightarrow steel and concrete reach their permissible stress at same time. & both material will fail at same time.

(b) Under reinforced secn :-

$x_u < x_{ubal}$ $A_{st} < A_{st\ bal}$

\Rightarrow steel failure.
 \rightarrow At some value of loads, stresses in steel is going to reach its permissible stress first & fail.

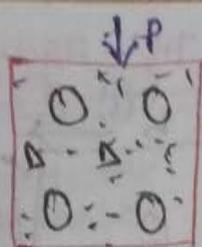
\Rightarrow Under reinforced secn is going under ductile failure

(c) Over Reinforced section :-

$x_u > x_{ubal}$ $A_{st} > A_{st\ bal}$

\Rightarrow At some value of loads, stress in concrete is going to reach first its permissible stress first & fails.

\Rightarrow This secn going to under Brittle failure



$A_c =$ Area of concrete

$A_s =$ Area of steel

Hook's law $\sigma \propto E$

$$\frac{P}{A} = \frac{\Delta l}{L} E$$

$$\Delta l = \frac{PL}{AE}$$

$$P = P_c + P_s$$

↓ load in concrete load in steel

Deformation of steel = Deformation of concrete

$$\Delta l_s = \Delta l_c$$

$$\Rightarrow \frac{P_c L}{A_c E_c} = \frac{P_s L}{A_s E_s}$$

$$\sigma_c = \frac{P_c}{A_c} = \sigma$$

$$\sigma_s = \frac{P_s}{A_s} = m \sigma$$

$$\Rightarrow \frac{\sigma_c}{E_c} = \frac{\sigma_s}{E_s}$$

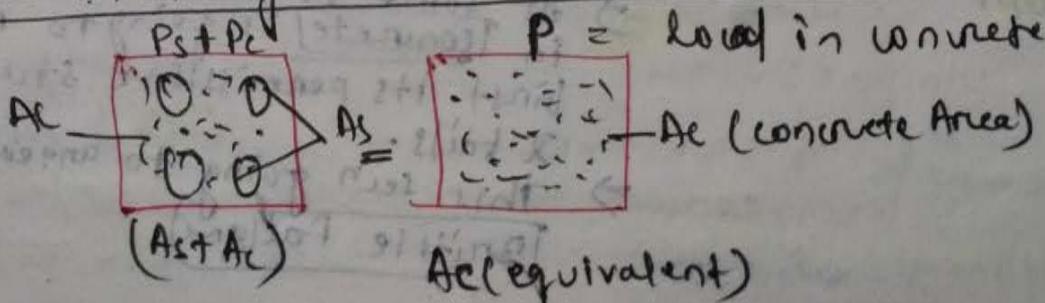
$$\Rightarrow \sigma_s = \frac{E_s}{E_c} \sigma_c$$

$$\Rightarrow \sigma_s = m \sigma_c \quad *$$

$$\therefore \frac{E_s}{E_c} = m$$

$$\sigma_c = \frac{\sigma_s}{m} \quad *$$

Stress in stronger material = $m \times$ stress in weaker material



Q1)

$$\mu_{bal} = \frac{m f_{cbc}}{m f_{cbc} + f_{st}} \times d \quad (or)$$

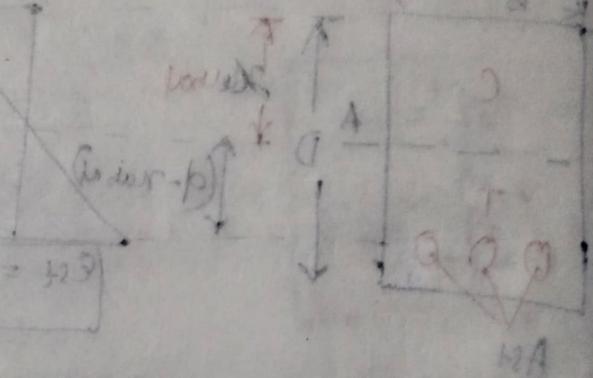
$$\mu_{bal} = \frac{1}{1 + \frac{f_{st}}{m f_{cbc}}} \times d$$

Q1) If modular ratio (m), effective depth is 'd' and stress ratio 'r' = $\frac{f_{st}}{f_{cbc}}$, the depth of neutral axis of a balanced section is

Soln:
$$\mu_{bal} = \frac{1}{1+r} \times d$$

- (a) $\frac{m+r}{m} d$ (b) $\frac{m}{m+r} d$ (c) $\frac{m-r}{m} d$ (d) $\frac{m+r}{r} d$

$$\mu_{bal} = \frac{1}{1 + \frac{3}{280} f_{st}}$$



Soln

$$\begin{aligned} & \frac{m f_{cbc}}{m f_{cbc} + f_{st}} \times d \\ &= \left(\frac{m}{m+r} \right) \times d \quad Ans \end{aligned}$$

Ans (B)

The depth of NA for a balanced section is dependent on

- (a) f_{cbc} (b) f_{st} (c) f_{cbc} & f_{st} (d) modular ratio.

Ans (B)

Q2) Which of the following sections performs better on the ductility criteria.

- (a) Balanced section (b) Over-reinforced section (c) Under-reinforced section (d) NOT

Ans (c)

13) The min^m cover in a slab should neither be less than the diameter of bar nor less than.

- (a) 10 mm (b) 15 mm (c) 25 mm (d) 13 mm Ans (B)

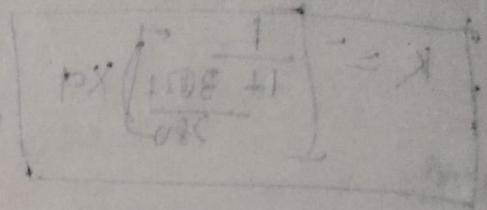
The recommended value of nominal or clear cover as per IS 456:2000

Slabs: - 15 mm

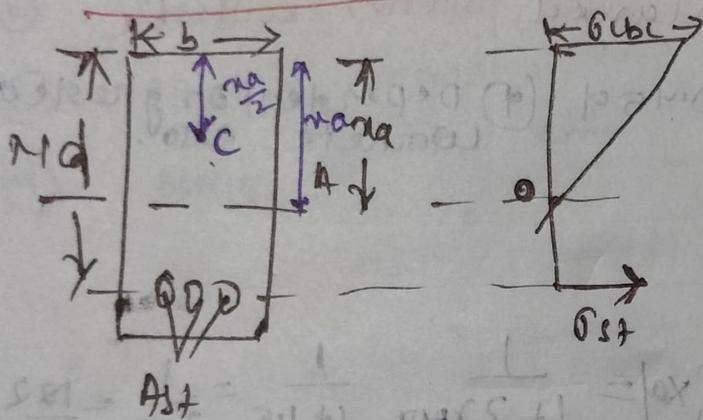
Beams: - 25 mm

Columns - 40 mm

Footing - 75 mm



2. calculate of actual depth of NA :-



$n_a =$ Actual depth of NA.

Moment of Area of compression side = Moment of Area in tension side, about NA

Moment = force \times distance

Moment of area of compression zone = $b \times n_a \times \frac{n_a}{2}$ about NA

force = $\frac{\text{force}}{\text{Area}} \times \text{distance}$

force = moment of area of compression zone